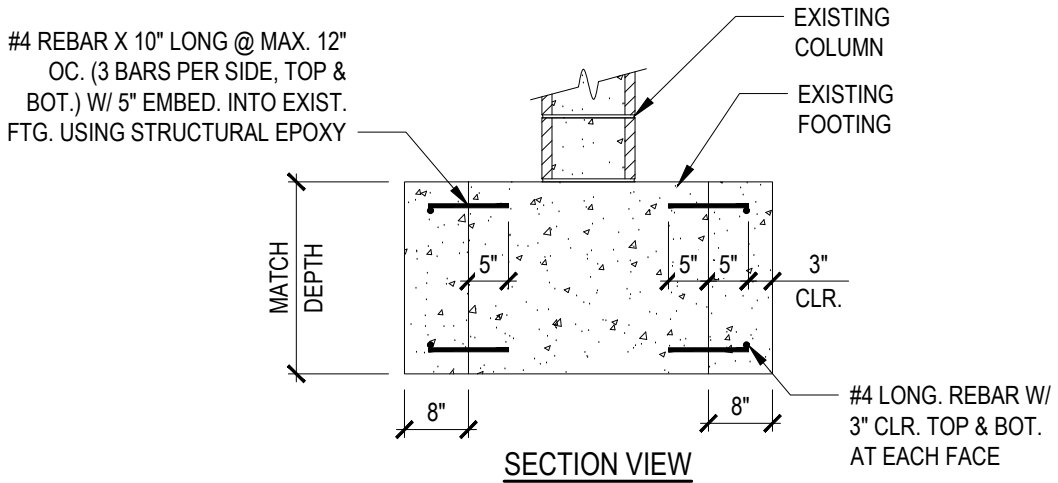
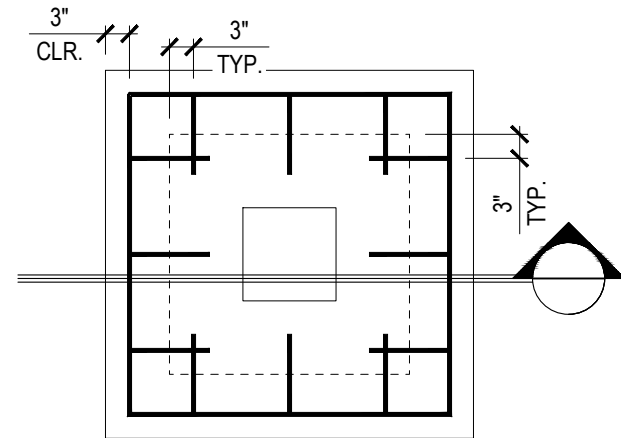
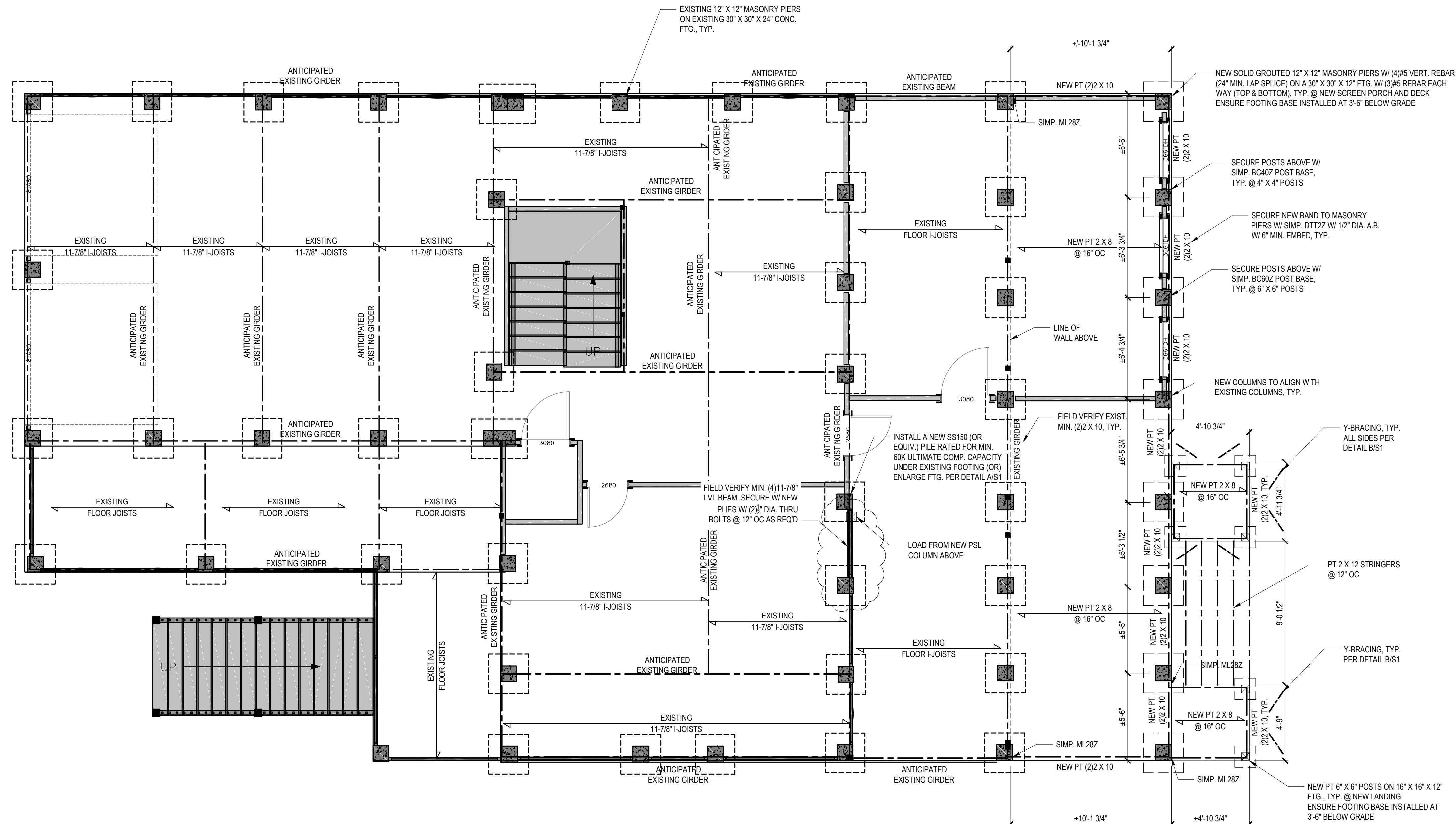


DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 150 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A-D			

STRUCTURAL NOTES:

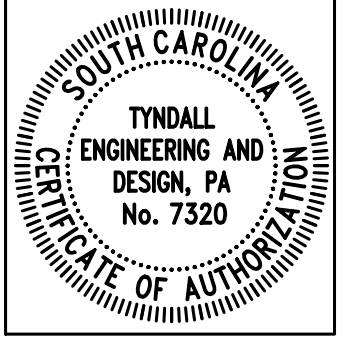
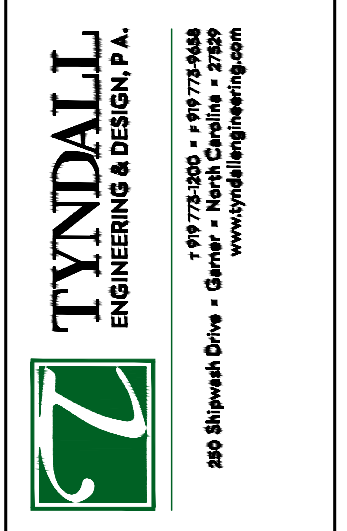
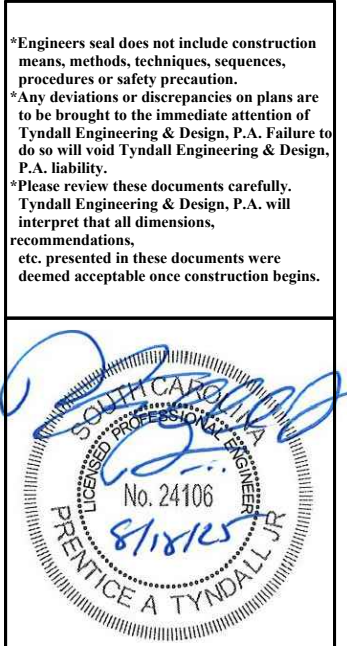
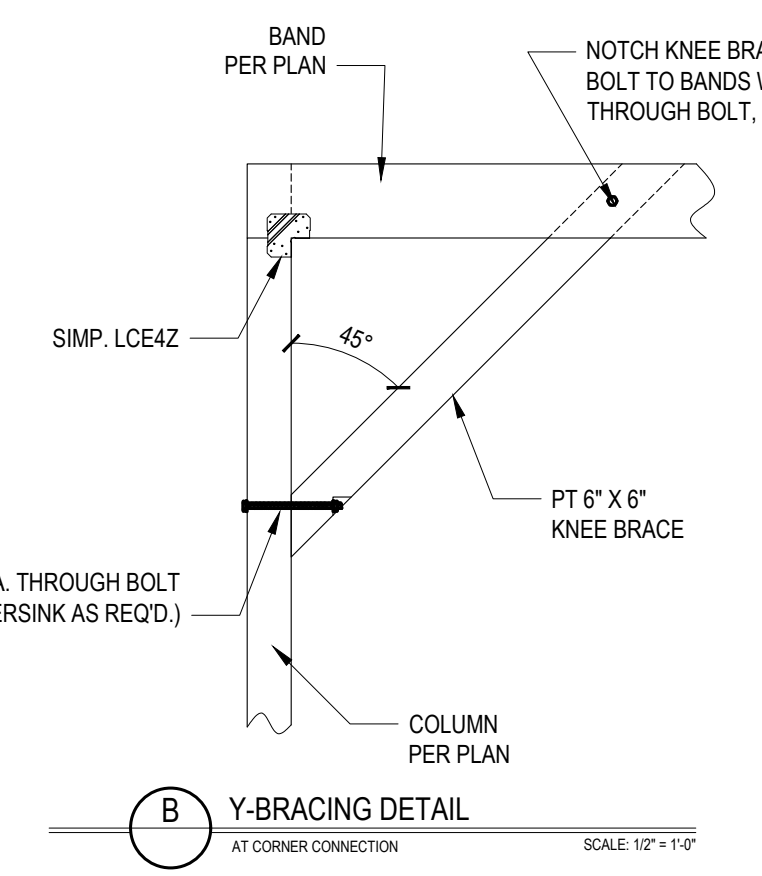
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "SOUTH CAROLINA STATE 2021 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
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- ALL LUMBER SHALL BE SYP #2 (UNO). ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND $F_b = 2600$ PSI, $E = 1.9M$ PSI (I.E. WEYERHAEUSER MICROLAM). ALL LSL LUMBER IS TO BE 1.55E ($F_b = 2325$ PSI).
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS SHOULD BE A (2)2x10 W/ (1)JACK STUD (MATCH WALL STUDS) AND KING STUDS PER THE SPECIFIED KING STUD CHART, AND NAILED TOGETHER W/ (2) 10d @ 8" O.C. (PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6", UNLESS NOTED OTHERWISE ON THE PLANS. OTHERWISE REFER TO TABLE R602.7(1). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (UNO) REFER TO TABLE R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO).
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 $F_y = 50$ KSI MIN. (UNO).
- ALL EXTERIOR LUMBER TO BE #2 SYP PT.
- ALL CONCRETE, $f_c = 4000$ PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF.
- 1/2" ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 6'-0" ON CENTER AND NOT MORE THAN 12" FROM CORNERS FOR (2)2X SILL PLATES. FOR (1)2X SILL PLATES, 1/2" ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 18" ON CENTER. A MINIMUM OF TWO BOLTS SHALL BE PROVIDED PER SILL PLATE SECTION. SILL PLATES SHALL BE ANCHORED WITH CONTINUOUS ANCHOR BOLTS EXTENDING INTO THE FOOTING, EACH WITH A 2"x2"x1/8" STEEL PLATE WASHER. ANCHOR BOLTS SHALL EMBED A MINIMUM OF 7" INTO TURNED-DOWN CONCRETE FOOTINGS.
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- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



NOTE:
FIELD VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION

NOTE:
THE EXISTING FRAMING SHOWN IS APPROXIMATE AND BASED ON LIMITED FIELD DATA AT TIME OF SITE VISIT. THE BUILDER SHALL FIELD VERIFY THE DIRECTION AND ACCURACY OF THE SHOWN FRAMING PRIOR TO DEMOLITION OR CONSTRUCTION. IF ANY EXISTING FRAMING DIFFERS FROM THAT SHOWN, THE BUILDER SHALL NOTIFY T.E.D. BEFORE PROCEEDING SO THAT THE PLANS CAN BE REVIEWED AND, IF NECESSARY, REVISED TO ENSURE STRUCTURAL ADEQUACY.

GROUND FLOOR PLAN
1/4" = 1'-0"



BETTER BY DESIGN
CLIENT: 3103 S SHORE DR
CHARLESTON, SC

**BASEMENT HEADER
1ST FLOOR FRAMING**

Project #: 2504-010004
Date: 08/18/25
Engineered by: AM
DWG. Checked by: PAT
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S1
1 of 5

DESIGN LOADS

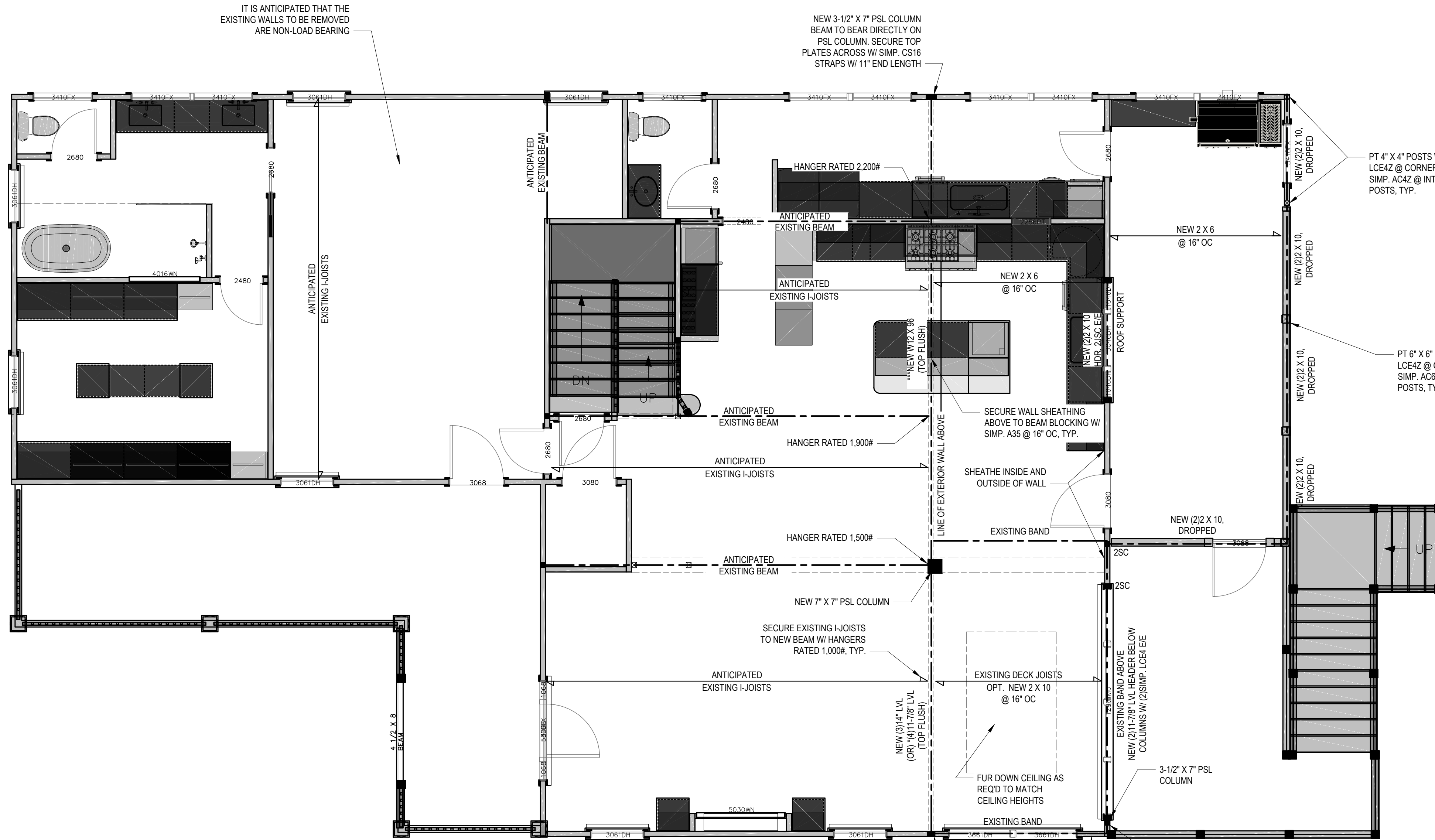
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no storage)	10	5	L/240	L/180
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ROOF	20	10	L/240	L/180
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WIND LOAD	BASED ON 150 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A-D			

STRUCTURAL NOTES:

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(I.E. WEYERHAEUSER MICROLAM)
ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS SHOULD BE A (2)x10 W/ (1) JACK STUD (MATCH WALL STUDS) AND KING STUDS PER THE SPECIFIED KING STUD CHART, AND NAILED TOGETHER W/ (2) 10d @ 8" O.C. (PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6", UNLESS NOTED OTHERWISE ON THE PLANS. OTHERWISE REFER TO TABLE R602.7(1). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (UNO) REFER TO TABLE R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
ALL CONCRETE, Fc = 4000 PSI MIN.
PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 1/2" Ø ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 6'-0" ON CENTER AND NOT MORE THAN 12" FROM CORNERS FOR (2) 2X SILL PLATES. FOR (1) 2X SILL PLATES, 1/2" Ø ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 18" ON CENTER. A MINIMUM OF TWO BOLTS SHALL BE PROVIDED PER SILL PLATE SECTION. SILL PLATES SHALL BE ANCHORED WITH CONTINUOUS ANCHOR BOLTS EXTENDING INTO THE FOOTING. EACH WITH A 2"x2"x1/8" STEEL PLATE WASHER. ANCHOR BOLTS SHALL EMBED A MINIMUM OF 7" INTO TURNED-DOWN CONCRETE FOOTINGS.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 10'-0" (UNO)
- PROVIDE A MECHANICAL CONNECTION WITH A MINIMUM 500# UPLIFT & LATERAL RATING AT TOP AND BOTTOM OF PORCH COLUMNS. (UNO)
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-D AND WIND SPEEDS OF 150 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH ICC600-2020 OR FEMA 55.
- PANELS SHALL BE APA RATED WOOD STRUCTURAL PANEL SHEATHING WITH AN EXPOSURE 1 BOND CLASSIFICATION UNLESS NOTED OTHERWISE. PANELS SHALL EXTEND OVER A MINIMUM OF THREE SUPPORTS AND SHALL BE INSTALLED IN FULL SHEETS EXCEPT AT ENDS OF WALL/ROOF SURFACES.
WALL SHEATHING MAY BE INSTALLED WITH STRENGTH AXIS PARALLEL OR PERPENDICULAR TO SUPPORTS. ROOF SHEATHING MUST BE INSTALLED WITH STRENGTH AXIS PERPENDICULAR TO SUPPORTS.
WALLS SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANELS USED TO RESIST BOTH SHEAR AND UPLIFT UNLESS NOTED OTHERWISE ON PLANS AND SHALL BE INSTALLED AS FOLLOWS:
- MINIMUM 7/16 THICKNESS
- PANELS SHALL BE ATTACHED TO FRAMING WITH 8d NAILS AS FOLLOWS (UNLESS NOTED OTHERWISE)
- 6" O.C. AT VERTICAL EDGES
- 6" O.C. AT INTERMEDIATE SUPPORTS
- 3" O.C. AT HORIZONTAL EDGES OR DOUBLE EDGE NAILING AT 3" O.C. WHERE REQUIRED. (NAIL SPACING IN ANY SINGLE ROW SHALL NOT BE LESS THAN 3" O.C.)
- PANELS SHALL EXTEND MINIMUM 12" BEYOND HORIZONTAL CONSTRUCTION JOINTS AND SHALL OVERLAP GIRDERS THEIR FULL DEPTH EXCEPT WHERE THE HORIZONTAL JOINT OCCURS OVER A MINIMUM 1" THICK OSB OR PLYWOOD RIMBOARD WITH A MINIMUM 1-1/2" OVERLAP
- BLOCKING SHALL BE PROVIDED AT ALL PANEL EDGES
- ALL REQUIRED UPLIFT CONNECTORS SHALL BE INSTALLED ON THE SAME SIDE OF THE WALL AS THE SHEATHING TO PROVIDE A CONTINUOUS LOAD PATH. CONNECTORS INSTALLED THROUGH SHEATHING SHALL BE ATTACHED WITH MINIMUM 2-1/2" LONG NAILS.
- INTERIOR SURFACES OF EXTERIOR WALLS AND ALL INTERIOR GYPSUM SHEAR WALLS SHALL BE SHEATHED WITH MIN. 1/2" THICK GYPSUM ATTACHED WITH 5d COOLER OR WALLBOARD NAILS @ 7" O.C.
HEADERS SHALL BE ATTACHED TO EACH JACK STUD AND EACH JACK STUD SHALL BE ATTACHED TO THE BAND OR RIM BOARD BELOW WITH A STRAP RATED FOR MIN. 1000 LBS.



NOTE:
ALL NEW LOAD BEARING HEADERS ARE TO BE (2) 2 X 10 PER STRUCTURAL NOTES 4 & 5, UNLESS NOTED OTHERWISE

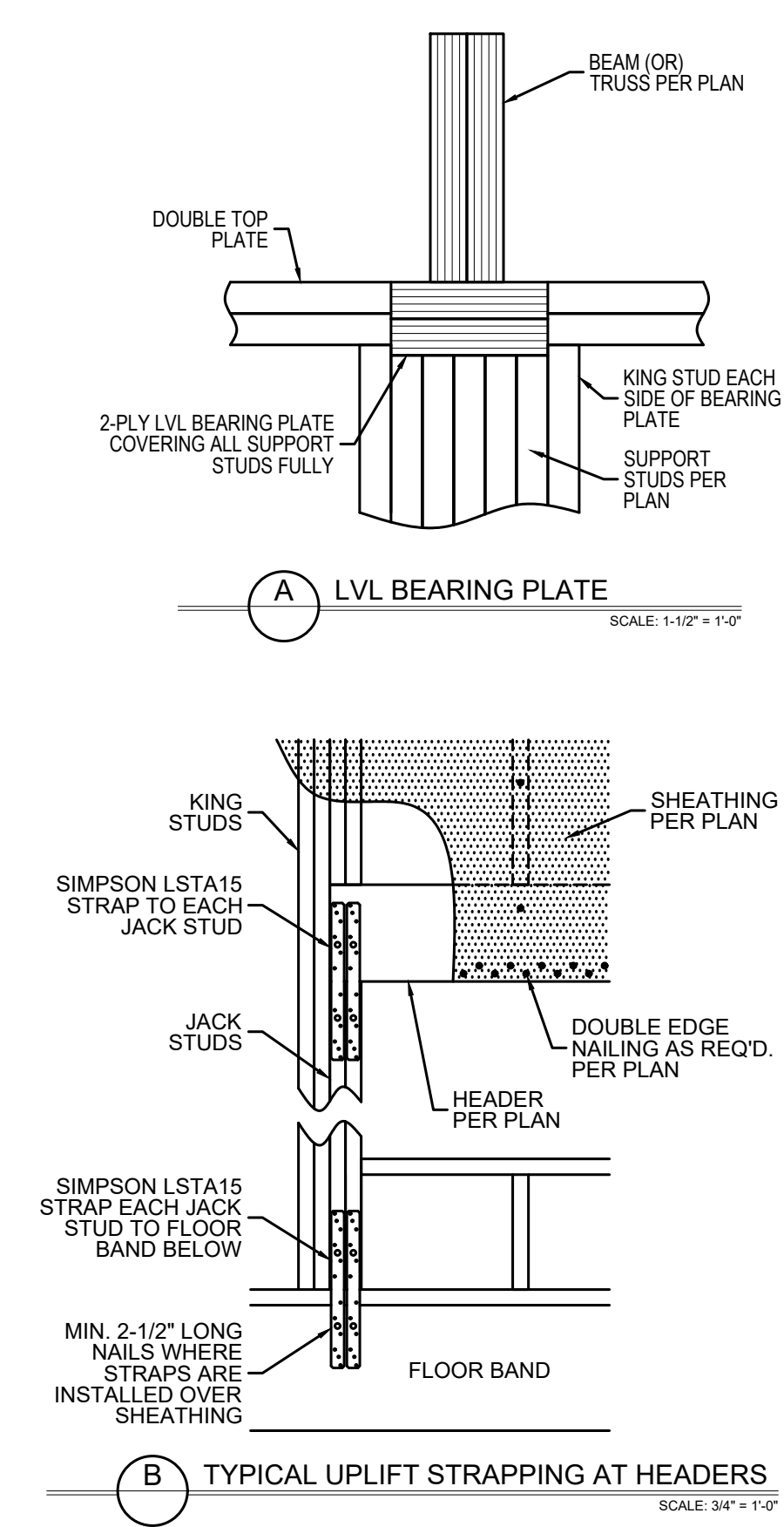
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NOTE: SECURE 4-PLY W/ 1/2" Ø THRU BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)

***W14 X 68, W16 X 57, AND W18 X 46 ARE ACCEPTABLE ALTERNATIVE STEEL BEAM SIZES

KING STUD SCHEDULE		
HEADER SPAN (FT)	MIN. # OF FULL HEIGHT STUDS (KING) E.E. OF OPENING PER WALL DEPTH	
	2 X 4 STUD WALL	2 X 6 STUD WALL
UP TO 3'-0"	2	1
3'-1" TO 6'-0"	3	1
6'-1" TO 9'-0"	3	2
9'-1" TO 12'-0"	4	2
12'-1" TO 15'-0"	6	3
15'-1" TO 18'-0"	7	3

NOTES:
a. TABLE DENOTES REQUIRED MINIMUM NUMBER OF STUDS E.E. OF HEADER, TYP. UNO ON PLANS
b. NUMBER OF KING STUDS LISTED ABOVE ARE BASED ON NOMINAL WALL HEIGHT, STUD SPACING OF 16" O.C. AND ULTIMATE WIND SPEED OF 150 MPH (EXPOSURE B)
c. HEADER SPAN IN TABLE BASED ON RUGH OPENING. INTERPOLATION BETWEEN SPAN VALUES IS PERMITTED. ROUND UP NUMBER OF KING STUDS. EXTRAPOLATION IS PROHIBITED. CONTACT TYNDALL ENGINEERING AND DESIGN, PA IF HEADER SPAN EXCEEDS TABLE VALUES
d. FOR BUILT-UP STUD ASSEMBLIES CONSISTING OF MORE THAN THREE STUDS, THE INDIVIDUAL MEMBERS SHALL BE SECURELY FASTENED TOGETHER TO ENSURE FULL COMPOSITE ACTION. FASTENERS (NAILS OR STRUCTURAL SCREWS) SHALL BE INSTALLED FROM BOTH SIDES OF THE ASSEMBLY IN A STAGGERED PATTERN AS REQUIRED



FIRST FLOOR PLAN
1/4" = 1'-0"

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TYNDALL ENGINEERING & DESIGN, P.A.
100 Blythewood Drive • Garner • North Carolina • 27828
www.tyndallengineering.com

TYNDALL ENGINEERING AND DESIGN, PA
No. 7320
3103 S SHORE DR
CHARLESTON, SC

BETTER BY DESIGN
Client: 2504-010004
Date: 08/18/25
Engineered By: AM
DWG. Checked By: PAT
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S2
2 of 5

DESIGN LOADS

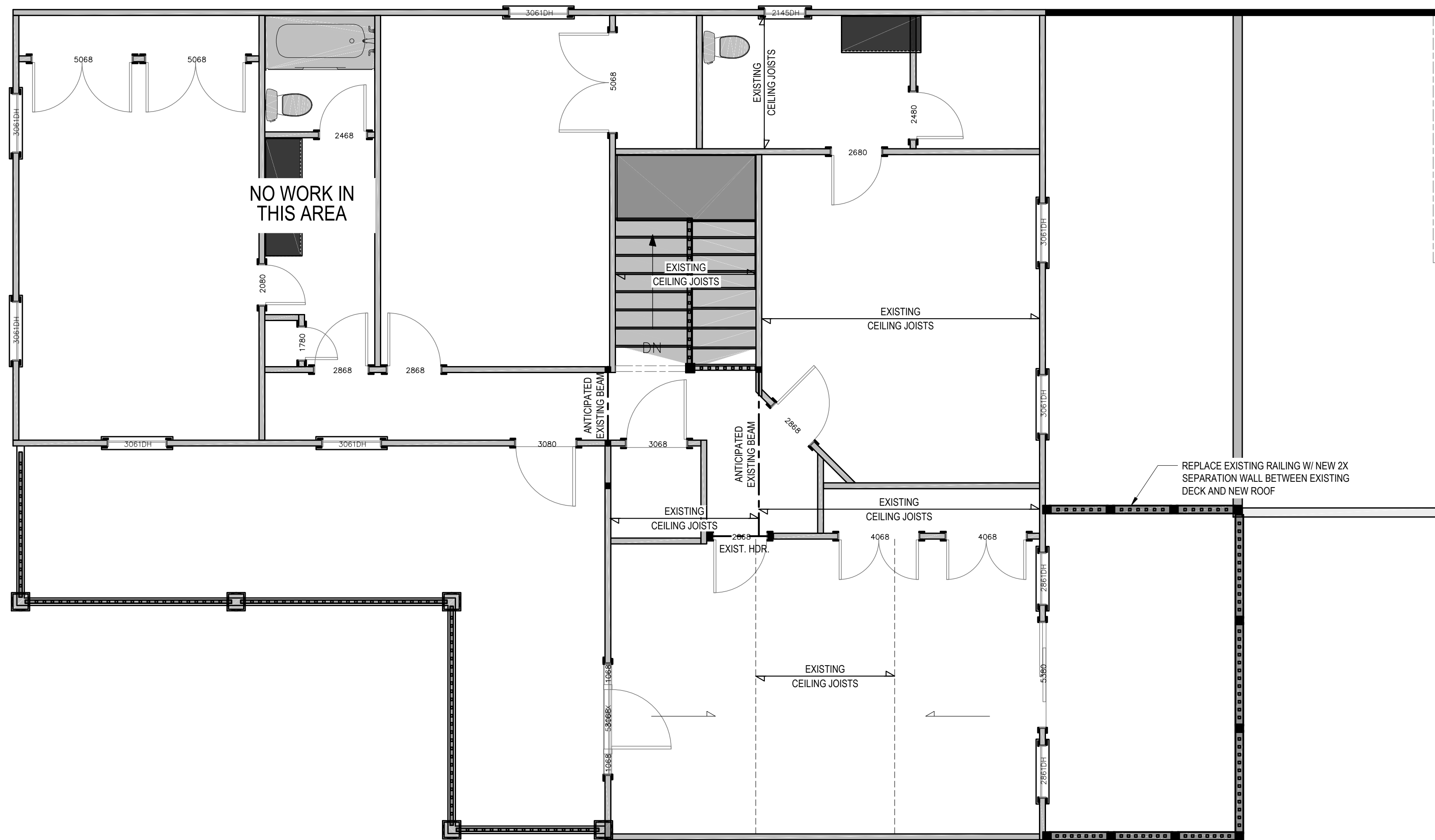
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- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, f'c = 4000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 1/2" Ø ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 6'-0" ON CENTER AND NOT MORE THAN 12" FROM CORNERS FOR (2)2X SILL PLATES. FOR (1)2X SILL PLATES, 1/2" Ø ANCHOR BOLTS SHALL BE SPACED A MAXIMUM OF 18" ON CENTER. A MINIMUM OF TWO BOLTS SHALL BE PROVIDED PER SILL PLATE SECTION. SILL PLATES SHALL BE ANCHORED WITH CONTINUOUS ANCHOR BOLTS EXTENDING INTO THE FOOTING. EACH WITH A 2"x2"x1/8" STEEL PLATE WASHER. ANCHOR BOLTS SHALL EMBED A MINIMUM OF 7" INTO TURNED-DOWN CONCRETE FOOTINGS.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 10'-0" (UNO)
- PROVIDE A MECHANICAL CONNECTION WITH A MINIMUM 50# UPLIFT & LATERAL RATING AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
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- MINIMUM 7/16 THICKNESS
- PANELS SHALL BE ATTACHED TO FRAMING WITH 8d NAILS AS FOLLOWS (UNLESS NOTED OTHERWISE):
- 6" O.C. AT VERTICAL EDGES
- 12" O.C. AT INTERMEDIATE SUPPORTS (6" O.C. WHERE LESS THAN 50 PERCENT OF WALL IS SHEATHED)
- 3" O.C. AT HORIZONTAL EDGES OR DOUBLE EDGE NAILING AT 3" O.C. WHERE REQUIRED. (NAIL SPACING IN ANY SINGLE ROW SHALL NOT BE LESS THAN 3" O.C.)
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SECOND FLOOR PLAN
1/4" = 1'-0"

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TYNDALL
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TYNDALL
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No. 7320
STATE OF SOUTH CAROLINA
REGISTERED PROFESSIONAL ENGINEER

BETTER BY DESIGN
3103 S SHORE DR
CHARLESTON, SC

2ND FLOOR HEADER
2ND FLR. CLG. FRAMING

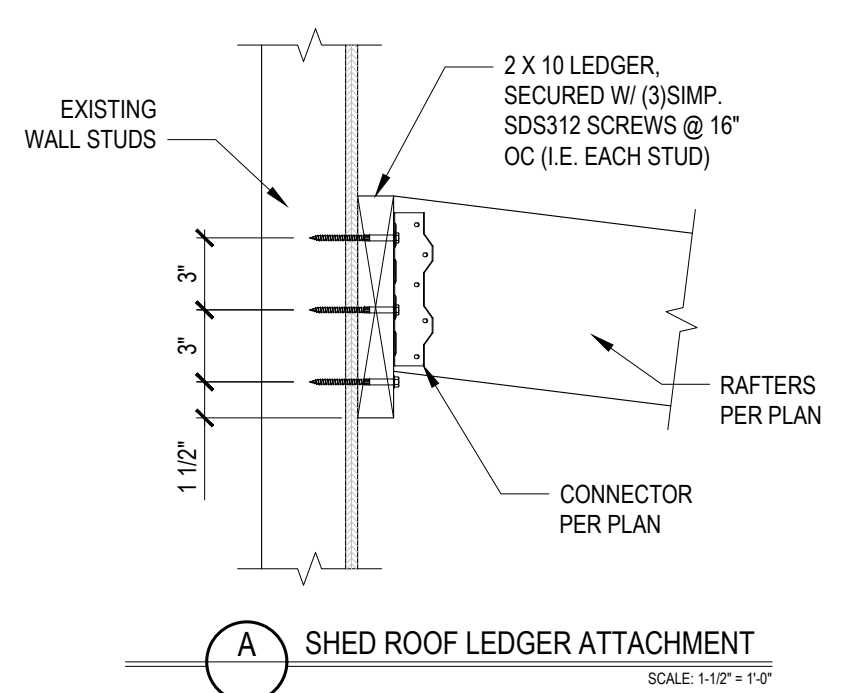
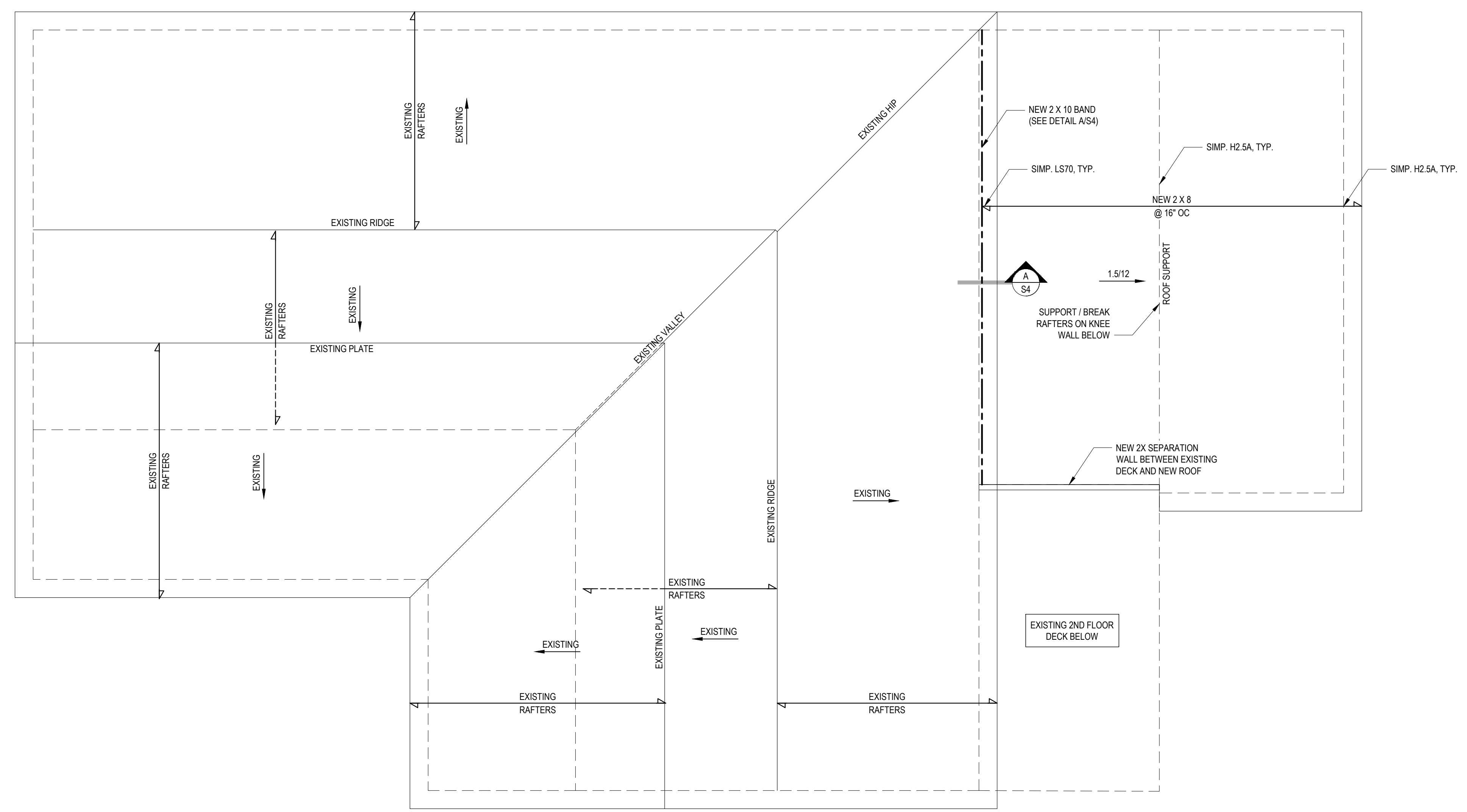
Project #:	2504-010004
Date:	08/18/25
Engineered by:	AM
DWG. Checked by:	PAT
Scale:	SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S3
3 of 5

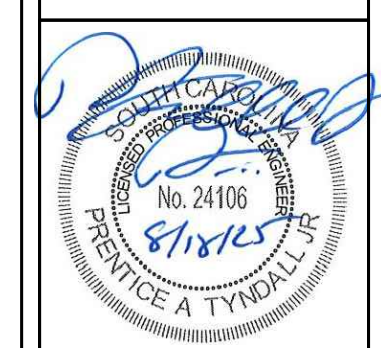
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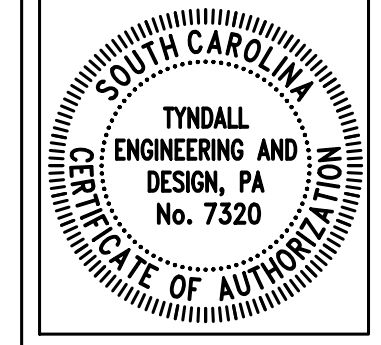


ROOF PLAN
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1107 73200 • 843 775 4444
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200 Blythewash Drive • Garner • North Carolina • 27820



BETTER BY DESIGN
3103 S SHORE DR
CHARLESTON, SC

ROOF PLAN

Project #:	2504-010004
Date:	08/18/25
Engineered By:	AM
DWG. Checked By:	PAT
Scale:	SEE PLAN

REVISIONS		
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STRUCTURAL NOTES

1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "SOUTH CAROLINA STATE 2021 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

2) DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			DL	LL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	60	10	L/360	L/240
ROOF	20	10	L/180	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	[BASED ON 150 MPH (ULTIMATE)]			

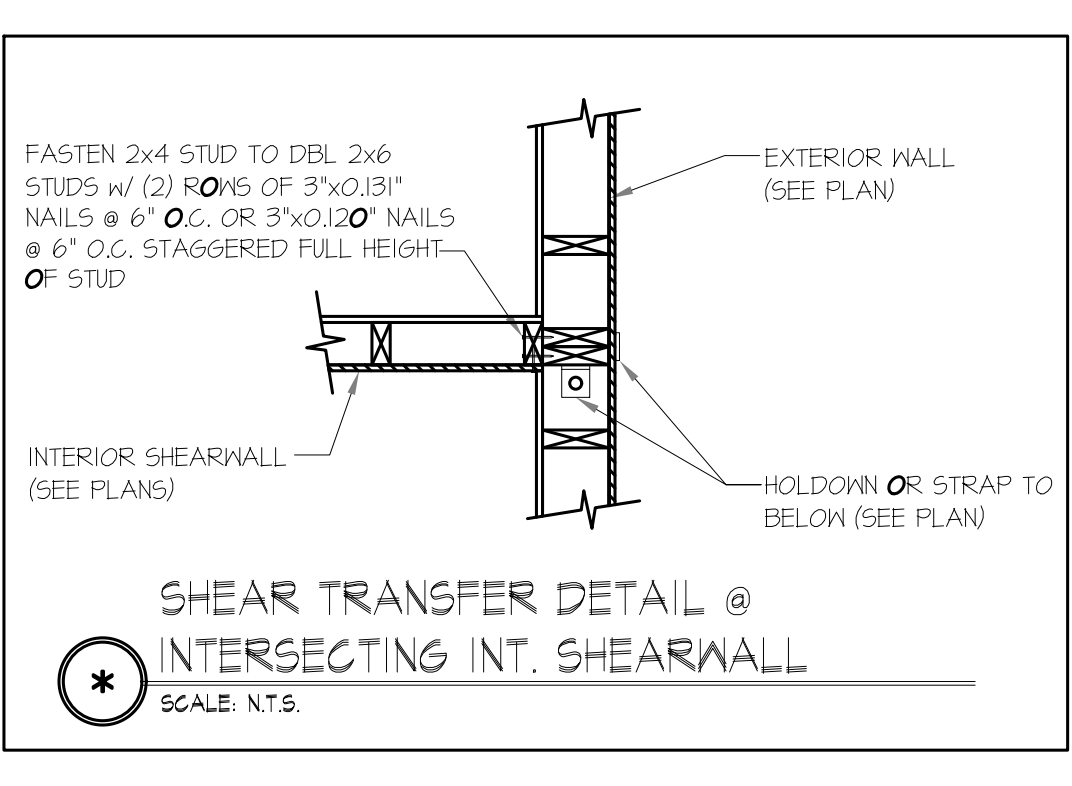
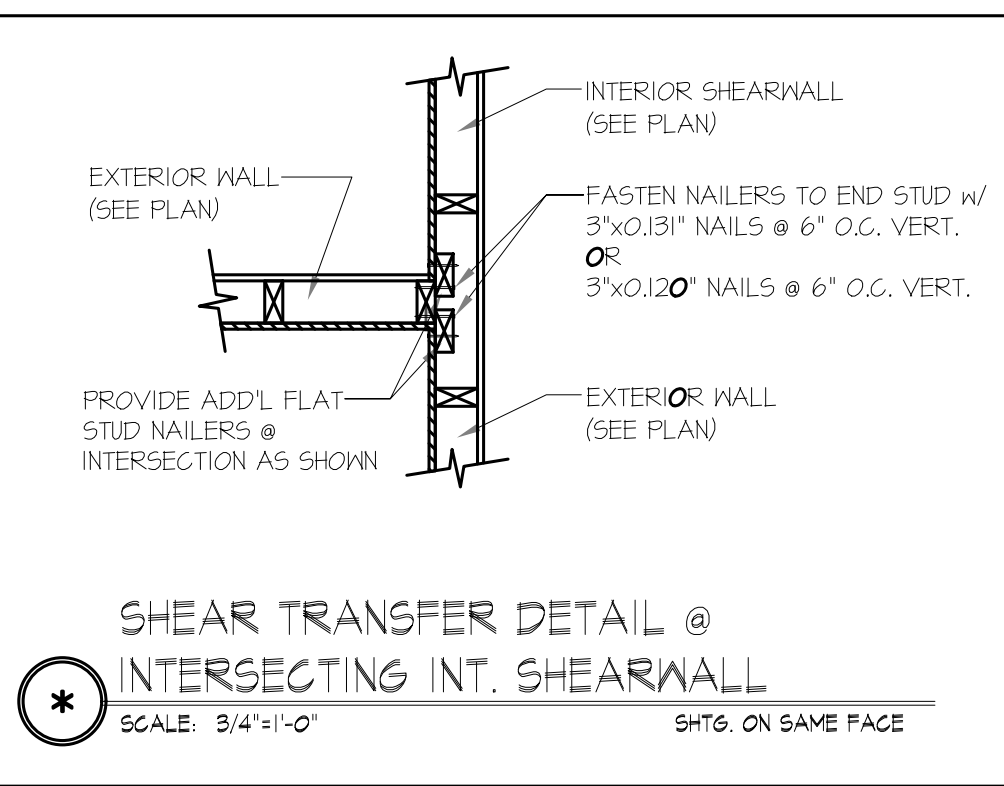
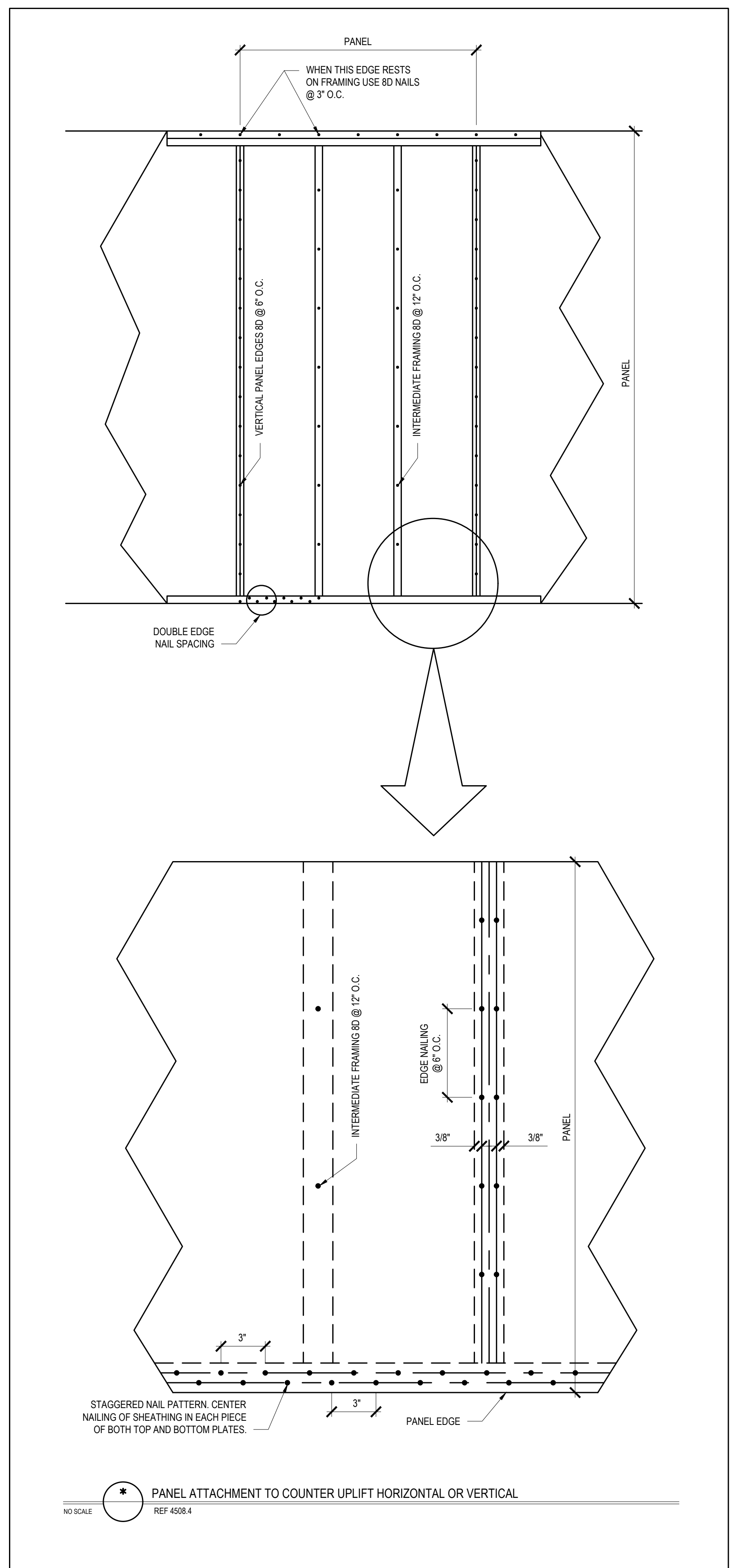
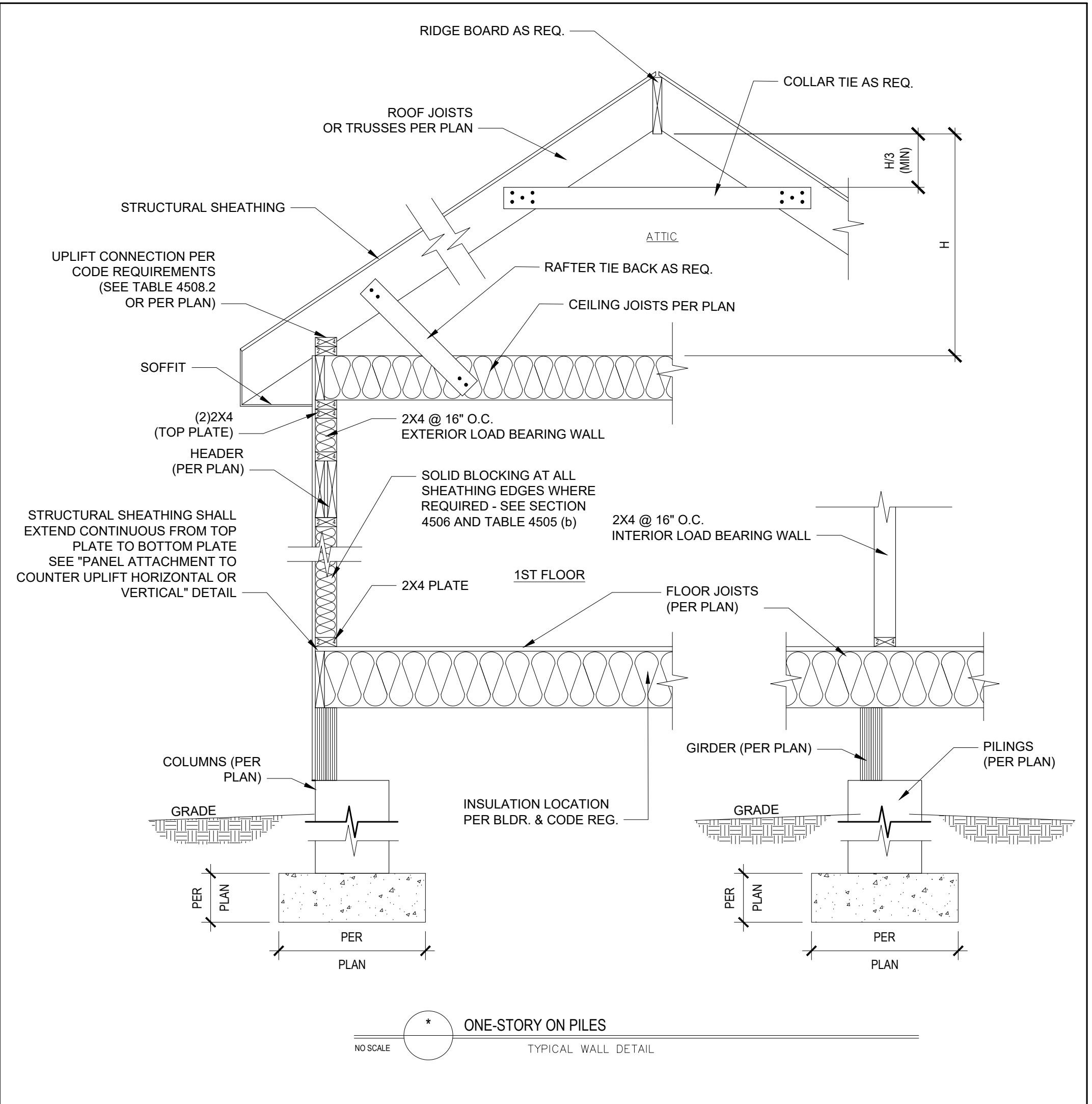
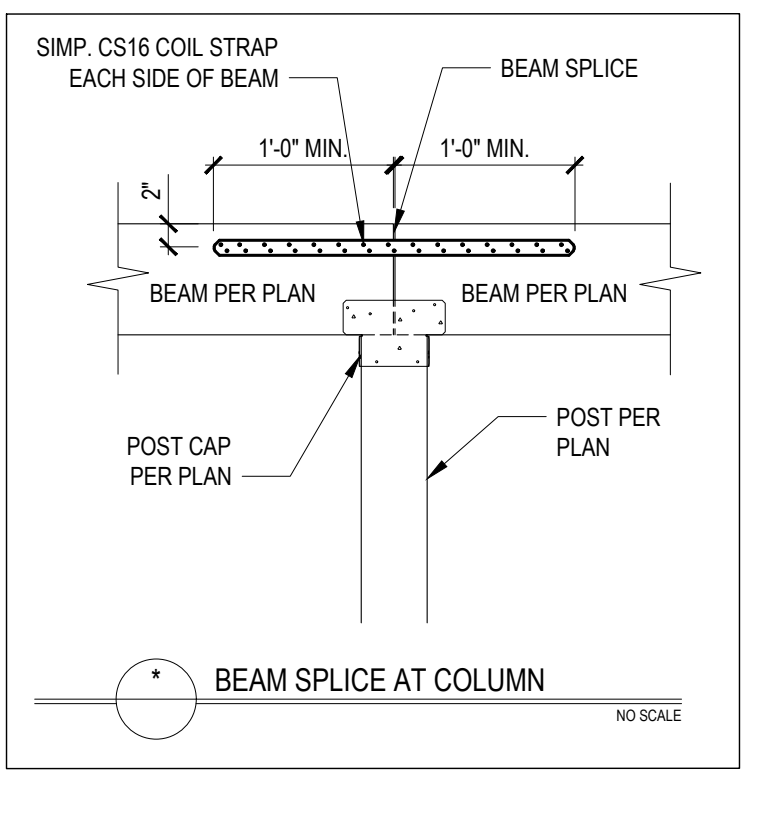
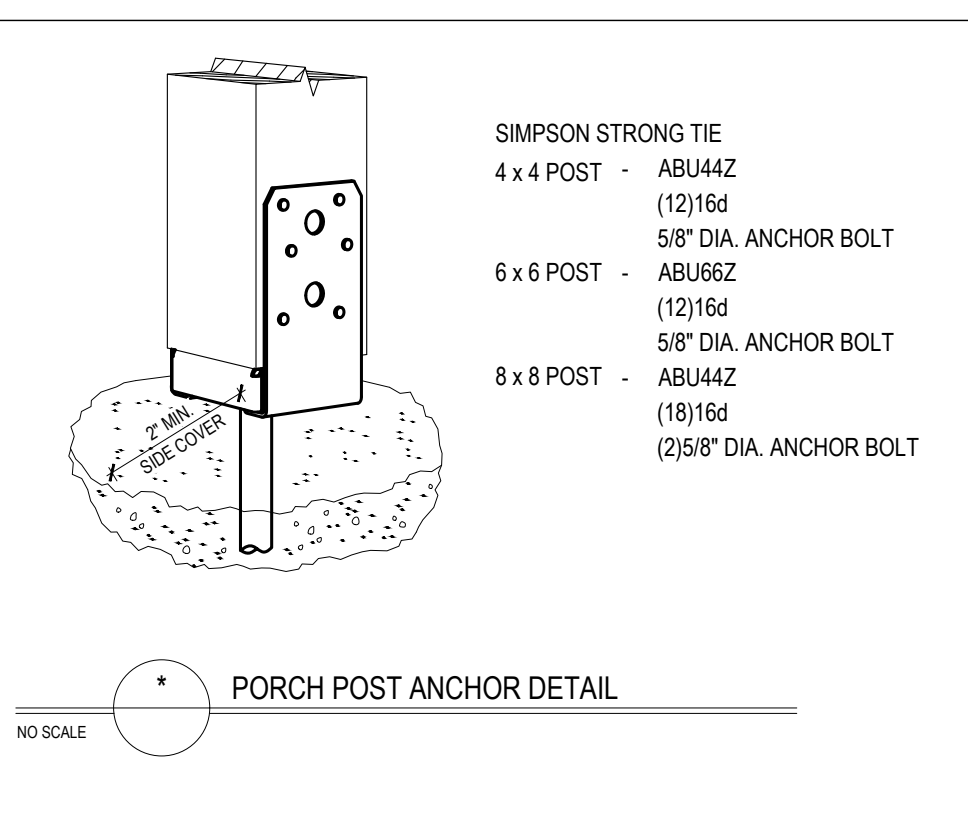
- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)
- 5) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2021 SC RESIDENTIAL CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- 6) ALL FRAMING LUMBER SHALL BE SYP #2 (FB = 800 PSI, BASED ON 2x10) UNO. ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL. ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.) ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.) ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- 7) ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10 (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- 8) ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- 9) STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2" x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 10) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF 2021 SC RESIDENTIAL CODE.
- 11) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 12) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 13) IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

HIGH WIND WALL BRACING NOTE:
ALL STORIES - WOOD STRUCTURAL SHEATHING PANELS REQUIRED.

GABLE ENDWALL BRACING NOTE:
GABLE ENDWALLS SHALL EITHER BE SUPPORTED BY LATERAL BRACING AT THE CEILING OR HAVE CONTINUOUS STUDS FROM FLOOR TO ROOF. 2x4 STUDS @ 16" O.C. ARE LIMITED TO 10'-0" IN LENGTH BETWEEN SUPPORTS. NON-BEARING 2x6 SYP #2 STUDS @ 16" O.C. w/ 7/16" WOOD STRUCTURAL PANEL SHEATHING ARE LIMITED TO 14'-0" (TYP).

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT = ALTERNATE	MANUF = MANUFACTURER
CANT = CANTILEVER	MAX = MAXIMUM
CJ = CEILING JOIST	MIN = MINIMUM
CMU = CONCRETE MASONRY UNIT	NOM = NOMINAL
COL = COLUMN	O.C. = ON CENTER
CONC = CONCRETE	PL = POINT LOAD
CONT = CONTINUOUS	PT = PRESSURE TREATED
CT = COLLAR TIE	REINF = REINFORCED
DBL = DOUBLE	REQ'D = REQUIRED
DIA = DIAMETER	RJ = ROOF JOIST
DJ = DOUBLE JOIST	RS = ROOF SUPPORT
DR = DOUBLE RAFTER	SC = STUD COLUMN
DSP = DOUBLE STUD POCKET	SCH = SCHEDULE
EA = EACH	SPEC = SPECIFIED
EE = EACH END	TH = THICK
FJ = FLOOR JOIST	TJ = TRIPLE JOIST
FND = FOUNDATION	TRTD = TREATED
FTG = FOOTING	TSP = TRIPLE STUD POCKET
GALV = GALVANIZED	TYP = TYPICAL
HORIZ = HORIZONTAL	UNO = UNLESS NOTED OTHERWISE
HT = HEIGHT	W = WIDE FLANGE BEAM
JSC = JACK STUD	WWF = WELDED WIRE FABRIC
KS = KING STUD	XJ = EXTRA JOIST



Engineers and designers are not responsible for construction methods, techniques, sequences, procedures or safety precautions. Any deviation or discrepancy on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

TYNDALL ENGINEERING & DESIGN, P.A.
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TYNDALL ENGINEERING AND DESIGN, PA
No. 7320
OFFICE OF AUTHORIZATION

BETTER BY DESIGN
3103 S SHORE DR
CHARLESTON, SC

STANDARD DETAILS

Project #: 2504-010004
Date: 08/18/25
Engineered by: AM
DWG. Checked by: PAT
Scale: SEE PLAN

REVISIONS

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